

VALORIZATION OF WASTE MACHINING OF STEEL PARTS: MANUFACTURE OF REINFORCED CONCRETE WITH CORRUGATED METAL FIBERS

BOUZEROURA Mansour¹, BOUAFIA Youcef²

Abstract

This work joins within the framework of the valuation of the waste of manufacturing "shavings" of the steel parts which we find in the industry; the concrete is a way of recycling of this kind of materials, indeed, these shavings play the role of the wavy metallic fibers, the presence of these shavings in a matrix concrete allows to give to the resulting composite a ductile behaviour and limit the distribution of cracks located by effect of sewing and absorption of energy. The modelling of the behavior in pulling of the reinforced concrete of wavy fibers (to variable geometry), is based on the study of the behavior of the fiber in the concrete matrix.

A solution to the problem of the modeling of the adhesion wavy fiber - concrete matrix was proposed bringing to light the evolution of the constraint of adhesion (τ_u) according to the "t" anchoring. This modeling is introduced in an effort relation-opening of the cracks, and then a passage to the law (constraint –deformation) was made by modifying this relation. Finally for every modeled parameter (adhesion, constrained-deformation in pulling and pickets submitted to a compound flexion), a calculation confrontation /test was been executed. The results of the confrontation are rather satisfactory.

Keywords: Concrete fiber, Fibers corrugated (wavy), Adhesive, Friction, Modelling

1. Introduction

Behavior of the composite (concrete fibers) is elastic linear until cracking of the matrix. The interior studies that one can quote [1], [2] and [5] show that at the instant where appears a crack there are a brutal decrease of the constraint.

We interest us in that follows to the behavior after cracking, therefore after the peak of load. In this domain, one considers the sliding phenomenon of fiber-matrix eventual plasticization of the fiber steel.

One expresses then the energizing equilibrium of a fiber at the time of the sliding.the used fiber.

¹ BOUZEROURA Mansour University of Abderrahmane Mira 06000 Bejaia Algeria, LaMoMS laboratory bouzeroura2013@gmail.com

² BOUAFIA Youcef University of Mouloud Mammerie 15000 Tizi Ouzou Algeria, LaMoMS laboratory Bouafia-y@yahoo.fr

To encircle well the contribution of fibers to the behavior of the composite in drive, it is essential to take into account the shape of the fiber (right, with anchorings in the extremities or still waved).

2. Modeling of the adhesion matrix in concrete Caption

2.1 The Equation of static equilibrium for an element of wire-drawn fiber

The wire-drawn fiber (said also to variable geometry) is decomposed in curves sections of curvilinear elementary length “ds”, Fig. 1. The equilibrium can be written in the following manner, Eq. (1), (2), (3) and (4). The projection of the forces along;

$$\text{The x axis: } P \sin\left(\frac{d\theta}{2}\right) + (P + dP) \sin\left(\frac{d\theta}{2}\right) = dN \tag{1}$$

$$\text{The z axis: } dP \cos\left(\frac{d\theta}{2}\right) = dT \tag{2}$$

For the small angle to elementary elements “ds”, we can do:

$$\sin\left(\frac{d\theta}{2}\right) = \frac{d\theta}{2}, \text{ and } \cos\left(\frac{d\theta}{2}\right) = 1$$

One ignoring the higher order terms $dP\left(\frac{d\theta}{2}\right)$, the Eq (1) and (2) become Eq (3) and (4). [5]

$$dN = Pd\theta \tag{3}$$

$$dP = dT \tag{4}$$

The equilibrium of an element of fiber at the time of the sliding, generated a radial effort “dN” and the tangential effort “dT”, that two efforts can be linked by a friction law of coulomb, this law consists to linking the shear stress “τ” to the normal stress “σ” of a facet by a friction coefficient “f” and a cohesion “C_o”. We will adopt a coulomb’s law in global forces, where the Eq (4) as Eq (5).

$$dT = f dN + \tau_o p ds \tag{5}$$

Where “p” is the perimeter of the fiber and “τ_o” is the bond stress, between the fiber and the matrix.

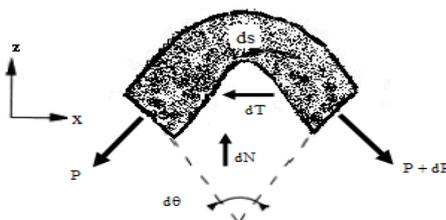


Fig. 1: Equilibrium of small element wire-drawn fiber [5]

2.2 Energy balance sheet during sliding

Physically at the time of the sliding, a fiber element undergoes, on the one part, a sliding of rigid body of amplitude “ $d\delta$ ” and on the other part, a variation of curvature “ dC ” produced by flexion. The fiber follows then the print of its initial geometry, the experimental observances of [5] showed that each section undergoes a plasticization. The purely statics approach of the fiber equilibrium does not permit an assessment of plastic energy dissipated during the sliding of the fiber. The equilibrium of this fiber element curve must be described from an energy point of view.

The balance of the mechanical energy, applied to the slipping elementary length curve of a fiber that, allows writing that the work of the external forces “ W_{ext} ” is equal to the work of the deformations “ W_{def} ”.

$$W_{ext} = W_{def} \quad (6)$$

The work of the external forces written as Eq. (7):

$$W_{ext} = \int_S p_i u_i dS \quad (7)$$

Correspond to the integration, of the product of the elementary forces “ $p_i dS$ ”, over the external surface “ S ” of the fiber elements, “ p_i ” is the pressure exerted on the exterior surface “ u_i ” are the kinematic displacement field of the element, “ i ” is the indication by representing a coordinate in the space. The strengths of mass, such as the gravity, are disregarded.

In the field of the displacements considered, only the components of the efforts (P , $P+dP$ and dT) hard working follow the curvilinear axis is taking in account Fig. 1. Disregarding the terms of higher order, the Eq. (7), written as Eq (8):

$$\begin{aligned} W_{ext} &= -P d\delta + (P+dP)d\delta - dT d\delta \\ W_{ext} &= dP d\delta - dT d\delta \end{aligned} \quad (8)$$

The work of the internal efforts is obtained by integration, over the element’s volume “ V ”, the product of the stress tensor “ σ_{ij} ” and strain tensor “ ε_{ij} ” defined in all dawned of the element’s volume Eq. (9).

$$\begin{aligned} W_{def} &= \int_V \sigma_{ij} \varepsilon_{ij} dV = \int_V \sigma z dC dV = dC \int_V \sigma z dV \\ W_{def} &= dC ds \int_S z \sigma dS = dC ds M \end{aligned} \quad (9)$$

With:

M : bending moment in the fiber,

ds : Elementary curvilinear length of the fiber.

dS : Straight section of the element’s fiber.

Considering the Eq (8), (9) the balance of the mechanical energy can be written in Eq (10):

$$dP d\delta - dT d\delta = M dC ds \tag{10}$$

Combining the Eq. (3), (5) and (10), we obtain the Eq. (11).

$$dP = (P f C + \tau_o p + M C') ds \tag{11}$$

Where “ C' ” is the first derivative of the curvature “ C ” against the curvilinear abscissa. The result of the integration of the Eq. (11) along the fiber gives a differential equation of the first order in “ P ”. It should be noted that the Eq. (1) to (12) were developed by Chanvillard [5].

In order to define the behavior’s problem of the wire-drawn fiber in the concrete matrix, the modeling of the fiber adhesion stamps (coefficient of rubbing) and the effect of the tilting of these fibers will do one of the principal aims of this study.

2.3 Modeling of the adhesion (coefficient of rubbing)

The coefficient “μ” is then defined starting from the isostatic equilibrium equations between the asperities in interaction, Fig. 2. Roughness rubbing has a sinusoidal profile, it results the following expressions.

$$\mu = (f + \chi)/(1 - f\chi) \tag{12}$$

$$\chi = \pi \frac{A}{\lambda} \cos \left[\pi \frac{\delta}{\lambda} \right] \tag{13}$$

With;

$\lambda/2$: Period,

χ : The slope of the contact,

A : Amplitude of the rough,

δ : Relative displacement of the rough and $\frac{A}{\lambda} \leq 0,015$, in [7].

In Eq. (13), the condition $\delta < \lambda/2$, drive at:

$$\left[\pi \frac{\delta}{\lambda} \right] < \pi/2 \Rightarrow \chi = 0 \quad \text{Where;} \quad \left[\frac{\delta}{\lambda} \right] < 1/2$$

Equations (12), (13) make it possible to plot the curve, Fig. 3, representing the evolution of “ μ ” according to the ratio “ $\frac{\delta}{\lambda}$ ”. This one makes it possible to fix the coefficient of apparent friction “ μ ”. Consequently, the variation of the tangential stress “ T ” given by the Eq. (5) is transformed into Eq. (14).

$$dT = \mu dN + \tau_o p ds \tag{14}$$

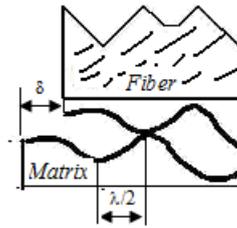


Fig. 2: Sinusoidal profile of the roughness rubbing

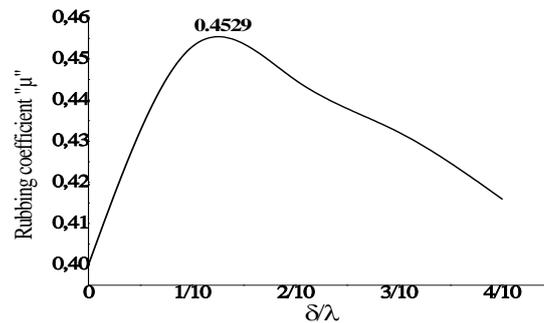


Fig. 3: Evolution of the visible rubbing coefficient μ
According to δ/λ

2.4 Determination of the probabilistic functions density for the case of orientation 2D of fibers in the matrix

The fiber is tilted of an angle “ θ ” compared to the axis of traction “ x_1 ”, Fig .4. This slope causes an effect of polished of friction and consequently, an increase in the forces compared to those of a fiber of null slope [3].

By referring to [7], the expression of the function of distribution “ G ” is given by the Eq. (15).

$$dG = (\phi, \psi) d\phi d\psi \tag{15}$$

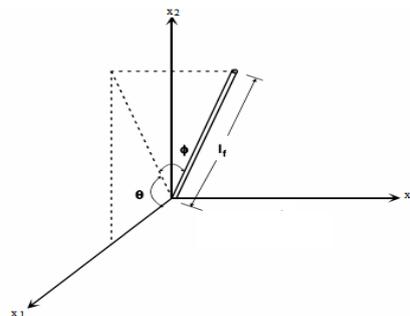


Fig. 4: Representation of an axis of wavy fiber in the space

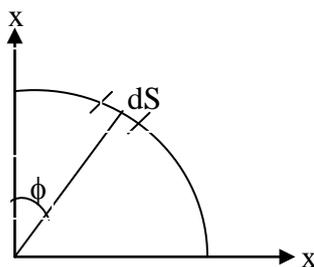


Fig. 5: Definition Space of the probabilistic density function (2D)

The fibers are directed arbitrary in the plan of the matrix concrete, in is interested only in the fibers which are in the plans of the force of pulling, in this case of orientation in 2 Dimension, Fig .5, with “ $\psi=0$ ”, the expression of “ $g(\phi,\psi)$ ” is transformed into “ $g(\phi)$ ”.

The determination of $g(\phi)$, into fixed a space of definition “ S ”, represented by a quarter of circle of ray unit. Probability that a point of “ S ” determined by the angle of orientation “ ϕ ” is on the part of “ S ”, noted “ ΔS ” is:

$$P_{rob} [g(\phi,\psi) \in \Delta S] = \frac{\Delta S}{S} = \int_{\Delta S} dG \tag{16}$$

$$\int_{\Delta S} dG = \frac{\Delta S}{S} = dG, \text{ With : } dG = (\phi,\psi)d\phi d\psi \quad \text{and} \quad dG = (\phi) d\phi = \frac{dS}{S} = \frac{1 \cdot d\phi}{2\pi \cdot 1}$$

$$(\phi) d\phi = \frac{2}{\pi} d\phi, \text{ where } g(\phi) = \frac{2}{\pi} \tag{17}$$

With 1: represents the unity ray.

The multiplicative factor is “ $e^{\mu\theta}$ ”, with “ μ ” coefficient of rubbing for the couple fiber matrix.

$$P_{\theta} = e^{\mu\theta} P_{\theta=0} \tag{18}$$

In axes system (x_1, x_2, x_3) , one can write; $\cos \theta = \sin \phi \cos \psi$ and we have Eq. (19);

$$e^{\mu\theta} = e^{\mu\theta \arccos[\sin \phi \cos \psi]} \tag{19}$$

The generalization of this expression to all fibers is done by application of the operator mathematical expectation of the function “ $g(\phi)$ ” given in Eq. (17).

$$\eta = \langle e^{\mu\theta} \rangle = \langle e^{\mu\theta \arccos[\sin \phi \cos \psi]} \rangle$$

For a two-dimensional and random distribution, one has; $g(\phi) = \frac{2}{\pi}$

$$\eta = \frac{2e^{\frac{\mu\pi}{2}}}{\mu\pi} (1 - e^{-\mu\frac{\pi}{2}}) \tag{20}$$

The influence of tilted fibers is then taken into account by the parameter “ η ”, Eq. (21).

Finally, the expression of the effort “ $P(s)$ ” necessary to pull the wire-drawn fiber of the matrix concrete, is given by the combination of the Eq. (10), (14) and (20), the Eq. (21) obtains some after integration over the length of the fiber [4].

$$P(s) = \eta \frac{\tau_o P + MC'}{-\mu C} (e^{\mu C s} - 1) \quad (21)$$

3. Confrontation forces displacements curve to experimental results

The developed model makes it possible to describe the behavior of a fiber undulated during its wrenching of a concrete matrix. It makes it possible to plot the curve effort of wrenching according to displacement. The confrontation of the curves obtained by the model suggested and the curves obtained on tests of wrenching carried out by [5], for various lengths of fiber ($\ell_f = 16$ and 24 mm), is given to the (Fig .6 and Fig.7).

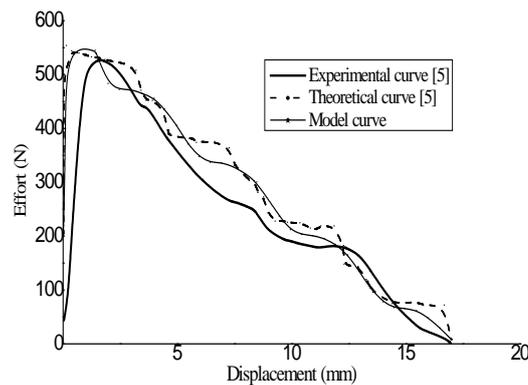


Fig. 6: Curve force – displacement ($\ell_f = 16$ mm)

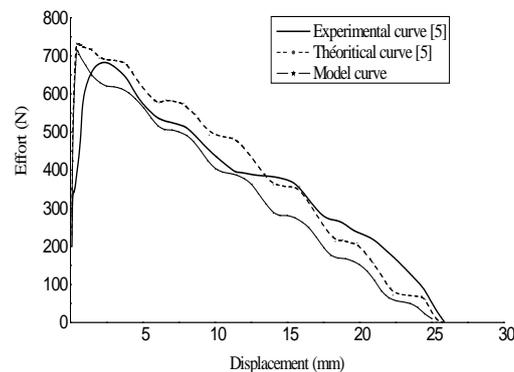


Fig. 7: Curve force – displacement ($\ell_f = 24$ mm)

The result of this confrontation is rather satisfactory. The shape of the experimental curves is correctly approximate. The model suggested is based on the application of the theorem of mechanical conservation of energy. The behavior is described using a first order differential equation. It utilizes friction with the interface matrix fiber of concrete, the

radial stress modifying those of shearing and the plasticization of steel. These phenomena are brought into play during the wrenching of corrugated fiber.

The geometry of fiber plays a dominating part in a test of wrenching. The fiber dissipates, during separation, an energy of friction in contact with the matrix (variable according to the intensity of the normal stress) and, at the time of the slip, an energy of plasticization result to the rectification of fiber. These two phenomena are taken into account. The behavior of the test-tubes is approximate in a rather satisfactory way.

4. Adaptation of the relations stress deformation suggested by [1] and [2] to wire-drawn fiber

4.1 Determination of the adhesion constraint τ_u of the undulating fibers

In order to determine an average constraint of adhesion matrix fiber, the calculation is executed for the three lengths of corrugated fibers quoted previously (8, 16 and 24 mm). This constraint is given by the expression (22).

$$\tau_u = \frac{P(s)}{\pi \phi la} \tag{22}$$

With;

ϕ ; Diameter of the fiber,

la ; adherent length of the fiber ($la = \ell_f \cdot t/2$),

t ; parameter of anchorage [1, 2].

$P(s)$; Effort of wrenching fiber,

The term “ t ” was the subject of a parametric study. The evolution of the constraint “ τ_u ” according to anchoring “ t ” is represented to the Fig. (8), for the three lengths.

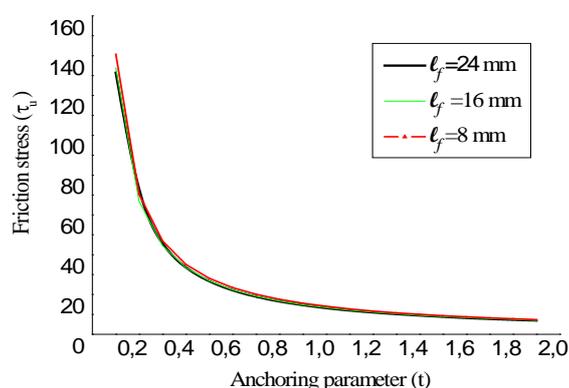


Fig. 8: The evolution of τ_u according to the parameter of anchorage t .

The value of τ_u adopted corresponds to the lower limit of the curve (with the stabilization of this one). When the value of “ t ” becomes higher than 1.4 and until to 2, the constraint of

adhesion “ τ_u ” does not evolve almost any more. The three curves converge towards the same value ($\tau_u = 7.92$ MPa).

This last value is then introduced into the relations stress-strains (23, 24) suggested by [1] and [2].

These relations are then modified for better taking into account the undulating fibers. From the expressions of opening of the cracks, the authors then made a passage to stress-strains relations; these are given by Eq. (23).

$$\left\{ \begin{array}{l} \sigma = E_{ct} \varepsilon \quad \text{si } 0 \leq \varepsilon \leq \varepsilon_{ft} \\ \sigma = \sigma_{uc} - [\sigma_{uc} - f_{ft}] \frac{(\varepsilon - \varepsilon_u)^6}{(\varepsilon_{ft} - \varepsilon_u)^6} \quad \text{si } \varepsilon_{ft} \leq \varepsilon \leq \varepsilon_u \\ \sigma = \sigma_{uc} \left[1 - \frac{(\varepsilon - \varepsilon_u)^6}{(\varepsilon_{rt} - \varepsilon_u)^6} \right] \quad \text{si } \varepsilon_u \leq \varepsilon \leq \varepsilon_{rt} \end{array} \right. \quad (23)$$

$$\left\{ \begin{array}{l} \sigma = E_{ct} \varepsilon \quad \text{si } 0 \leq \varepsilon \leq \varepsilon_{ft} \\ \sigma = \sigma_{uc} - [\sigma_{uc} - f_{ft}] \frac{(\varepsilon - \varepsilon_u)^6}{(\varepsilon_{ft} - \varepsilon_u)^6} \quad \text{si } \varepsilon_{ft} \leq \varepsilon \leq \varepsilon_u \\ \sigma = \alpha \sigma_{uc} \left[1 - \frac{(\varepsilon - \varepsilon_u)^\beta}{(\varepsilon_{rt} - \varepsilon_u)^\beta} \right] \quad \text{si } \varepsilon_u \leq \varepsilon \leq \varepsilon_{rt} \end{array} \right. \quad (24)$$

Tab. 1: Mechanical characteristics of the composite
(Concrete reinforced with steel fibers)

f_{ci}	f_{ti}	R_c	ε_{rt}	ε_{cu}
(MPa)			‰	
47,7	2,94	0,7	-50	3,5

Tab. 2: Characteristics of the fibers

τ_u	E_f	ℓ_f	ϕ	ϖ	ε_u
MPa	GPa	mm		“%”	“‰”
7,92	200	60	1	0,31	- 0,74

The effort of pulling and the shear stress are given respectively by the Eq. (21) and (22). According to these considerations the Eq. (23) are transformed into Eq. (24). They thus make it possible to take account of the corrugated geometrical shape of fiber. [3].

With;

- σ_{uc} ; Maximum ultimate stress,
 - ε_{ft} ; Deformation of the concrete cracking,
 - ε_{rt} ; Rupture of the composite tensile shear,
 - f_{ft} ; Tensile strength of the composite,
 - E_{ct} ; Initial module of composite tensile,
 - ε_u ; Strain corresponding to the total mobilization, of the fiber matrix adhesion.
- $\alpha = 0,8$, $\beta = 7$.

The parameters “ α ” and “ β ” are given starting from the confrontation of the results Eq. (24) to experimental results. The model suggested is introduced into nonlinear computation software, until rupture, of a section of beam “CMP”. [6].

4.2 Confrontation of the relation stress-strain in direct pulling

These relationships comparison is made to the results obtained on cylindrical specimens tested at direct tensile by Zhan [8], these specimens of metallic fibers wavy reinforced concrete are BFO25 and BFO40 respectively determined to 25 kg/m^3 and 40 kg/m^3 . The mechanical characteristics of concrete and fibers are given in tables (1 and 2).

With; f_{ci} ; compressive resistance of the composite,

- f_{ti} ; Tensile strength of the composite,
- R_c ; Ductility material coefficient
- ε_{cu} ; Concrete compressive breaking strain,
- E_f ; Elastic fiber module,
- ϖ ; Percentage by volume of fiber,

The specimens Strains tested have been obtained by dividing the crack measured by the length of the extensometers, fixed on the specimen (140 mm). The comparison of the results is illustrated in Fig. 11, and detailed in Fig. (12).

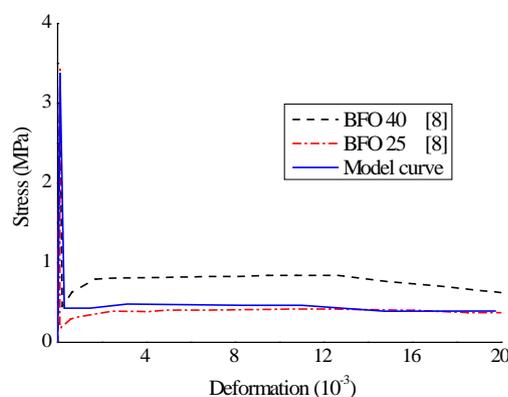


Fig. 09: Comparison with the results obtained by [8] (Fiber length $l_f = 60 \text{ mm}$)

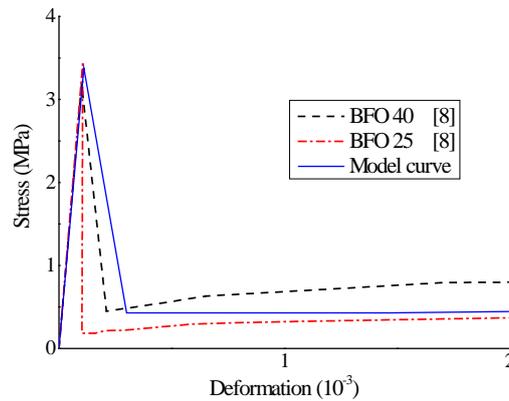


Fig. 10: Detail of figure 11

According to these results, there is a phase before the break which is elastic linear, the tensile strength is substantially the same between the two test pieces and the proposed model, then there is a sharp drop in effort which looks like the rupture of the composite and a residual strength is maintained in the final phase.

Finally, up to a strain of approximately 10^{-2} , the load tends to decrease. It can be concluded that the behavior is approached in a satisfactory manner.

5. Conclusion

The incorporation of steel fibers in a matrix concrete gives the composite a ductile behavior in traction. By the effect of sewing of the cracks, the fibers limit their spread and absorb a certain quantity of energy.

This phenomenon is a function of the geometry and the mechanical characteristics of the fiber used. In order to determine the contribution of fibers well to the behavior of the composite in traction, it is essential to take account of the shape of fibers (right, with or without anchoring at the ends or corrugated). Within the framework of this work, one was interested in corrugated fiber. Modeling is based on the effect of rubbing and friction and takes into account the undulations of the fiber.

A parametric study made it possible to model the friction of a fiber undulated in a concrete matrix and to highlight the evolution of the friction stress " τ_u " according to the parameter of anchoring " t ". This modeling is introduced by adaptation of the stress-strain relations suggested for a concrete reinforced with fibers in traction by [1] and [2].

The software, making it possible to follow the nonlinear behavior until the rupture of a concrete section, proposed by [7], is used to validate this modeling. The results of the confrontations carried out are satisfactory.

References

- [1] Bouafia. Y, Kachi. M.S, Foure. B, “Relation Effort - Opening of the Cracks in the Case of Concrete Reinforced with Steel Fibers”, Annals of the ITBTP, France, vol. 02, pp. 5–14, Apr. 1998.
- [2] Bouafia. Y, Kachi. M.S, Foure. B, “Relation Stress-strain in Traction of the Concrete Reinforced with Steel Fibers”, Annals of the ITBTP, France, vol. 03, pp. 15–19, June, 2002.
- [3] Bouzeroura. M, Kachi. M.S, Bouafia. Y, “Modeling of the Behavior of the Concrete Reinforced with Corrugated Metal Fibers”, Mediterranean conference on materials (CEMEDIMAT), Oran, pp. 22–34, December 6, 7, 2005
- [4] Bouzeroura. M, Kachi. M.S, Bouafia. Y, “Proposal for a Model of behavior at pulling of the reinforced concrete of waved fibers”, 5th International Conference Fiber Concrete 2009. pp. 43–52, Prague 17th -18th September 2009
- [5] Chanvillard. G, “Modeling the pullout of wire-drawn steel fibers”, Cement and concrete research, vol. 29, pp.1027-1037, 1999
- [6] Kachi. M.S, “Nonlinear calculation until rupture, of a Concrete section reinforced with metal fibers”, Master. Dissertation, Dept. Civil. Eng., Tizi-Ouzou University, Algeria, 1997.
- [7] Douglas. M. AN, “Modeling of the mechanical behavior of the composites cements fibers by taking account of the microstructure”, PhD dissertation, I.N.S.A of Lyon, 1997.
- [8] Zhan. Z, “Contribution to the dimensioning of the fiber Concrete foundation piles”, PhD. Dissertation, University of Orleans, 1991